SE INSIGHTS
FOR MASONRY DESIGN
Masonry Checklist: Reviewing Structural Plans
\Box f' _m (masonry assembly strength) for structural concrete or clay masonry is 2,000 psi or greater
Concrete masonry f'_m = commonly 2,500 psi or above
Clay masonry f'_m = commonly in the range of 3,000 psi to 4,000 psi
Masonry strengths up to 4,000 psi are permitted in current codes for strength design ¹
Check that all components are specified - SE INSIGHT: HOW TO SPECIFY MASONRY6
Block strength: check <u>masonry.forsei.com/masonry/cmudata/</u> to verify based on location
Commonly above 3250 psi for concrete masonry and 8250 psi for clay masonry
\Box Mortar type (mortar strength need <u>not</u> be listed)
Recommend Type N for non-structural walls
Veneer walls commonly use this mortar
Can be used in some structural applications, but reduces capacity
Not to be used below grade
□ Not to be used in seismic SDC D, E, or F
Recommend Type S for structural walls
Can be used below grade
Can be used in all seismic areas, SDC A, B, C, D, E, and F
Type M is high strength, but more costly and reduced workability
Can be used below grade
Used in high load applications and extreme environmental conditions
Grout strength
\Box Should be at least 2,000 psi, and equal to or greater than f' _m

Consider masonry wall thickness and reinforcing
 Reinforced masonry walls can be designed to have a height:thickness ratio up to 30:1 (common) or 50:1 (higher strength and heavily reinforced) 4" hollow clay masonry walls can be 10 ft to 15 ft tall
6" concrete or hollow clay masonry walls can be 15 ft to 25 ft tall
8" concrete or hollow clay masonry walls can be 20 ft to 33 ft tall
[] 10" concrete or hollow clay masonry walls can be 25 ft to 42 ft tall
12" concrete masonry walls can be 30 ft to 50 ft tall
[] 16" concrete masonry walls can be 40 ft to 67 ft tall
Consider the following for walls as well:
\Box 4" is available for concrete and clay; only 4" hollow clay masonry can be reinforced
\Box 6" - 8" - 10" are available for concrete and hollow clay masonry can be reinforced
12" - 16" are available for concrete only and can be reinforced
 Notes: Guidelines in TMS 402 code¹ recommend limiting reinforcement to:
 Less than 1/8 of the overall wall thickness
 Strength Design (SD): Less than 1/4 of the least clear dimension, less than 4% of the cell area, and maximum bar size of #9
 Allowable Stress Design (ASD): Less than 1/2 of the least clear dimension, less than 6% of the cell area, and maximum bar size of #11
Reinforcement specified in schedule
\Box Typical walls have reinforcement bars ranging from #4 (min) through #7 (max)
Reinforcement in columns/piers can be larger, but generally not more than #9
\Box Lap lengths are specified for correct f' _m and based on current TMS 402 code ¹
Bond beam and joint reinforcement specified and coordinated with CJ locations
Review masonry shear walls - masonry walls are effective to resist lateral loads
Verify masonry walls are considered and designed for lateral load
 Note: Masonry walls connected to the structure diaphragms will resist substantial portion of lateral loads due to the stiffness and rigidity of masonry

Wall groups (4 walls connected at corners, shaft walls) are more effective than 4 individual walls (separated by CJ) for lateral load resistance
Masonry shear walls can be unreinforced, but that is uncommon for new construction
Masonry reinforced shear wall designations:
Ordinary Reinforced Walls (summary)
Vertical reinforcement within 16" of opening edge and within 8" of wall ends
Minimum of 2-W1.7 horizontal joint reinforcement at 16" o.c.
Alternate: #4 @120" o.c. horizontal
Reinforcement above and below openings greater than 16" wide
Extend reinforcement 24" past opening, nor less than 40db
Reinforcement within 16" of top of walls
Intermediate Reinforced Walls, all requirements of Ordinary Reinforced
\Box vertical reinforcement no further apart than 48" o.c.
Special Reinforced Walls, all req. of Ordinary and Intermediate Reinforced
Reinforcement no further than 48" o.c. vertically and horizontally
• Note: see current TMS 402 code ¹ for many additional requirements
Review masonry partition wall designs and connections based on SE INSIGHT: MASONRY PARTITION WALLS ⁷
• Consider: the code prohibits partitions from serving a structural function for the building and therefore can have minimal reinforcement - see <u>imiweb.org</u> for partition wall program.
Partition walls can still be unreinforced in SDC A and B with minimal connections
For higher seismic areas, min. reinforcement:
\Box SDC-C - #4@120"0.c.
\Box SDC-D - #4@48"o.c.
SDC-E & F, see TMS 402 code

Verify that movement joints (MJs) are located - control joints (CJs) are common for structural concrete masonry and expansion joints (EJs) are common for structural clay masonry. General nomenclature is to use either MJ for both, or CJ (concrete masonry) and EJ (clay masonry).
CJs or EJs for structural walls <u>must be located</u> on structural elevations or plans ¹
CJs or EJs in <u>reinforced</u> structural walls, locate:
At common wall locations ² : generally at 25 ft spacing or less, change of wall height, building corners
At a distance (recommend 2 ft) away from opening edges ³ , not at opening edges
CJs or MJs in <u>unreinforced</u> non-structural masonry walls, locate:
At common wall locations ²
At openings edges 4
CJs or MJs not needed when sufficient horizontal reinforcement 5 is provided
Review lintels based on SE INSIGHT: EFFICIENT MASONRY LINTELS ⁸ ; prefer masonry lintels.
Masonry lintels are considered for ALL openings, other materials as optional
Openings that do not need a lintel:
\bigcirc 8" in typical wall, 12" in partition wall ¹
Openings 6'-0" or less could be a single-course masonry lintel with minimal reinforcement, and jamb could be one cell with common wall reinforcement
Openings more than 6'-0" are likely multi-course masonry lintels
Bottom masonry course is lintel block
Top masonry course is bond beam block
If middle courses are necessary, standard wall block can be used
• Note: only consider stirrups when deeper lintels are not possible
• Consider: prefab masonry lintels (contractor option) to resolve shoring needs
Optional: Precast concrete lintels can be similar to masonry lintels
Prefer precast lintels shaped like lintel u-block, reinforced with tendons
Does precast lintel lintel allow for jamb reinforcement to be continuous?
Does lintel allow for bottom and top reinforcement that can be tied into the wall?

Optional: Steel lintels- present many challenges
 Consider: differential movement between steel and masonry, even after building is insulated and occupied, will cause very large forces unless steel is allowed to move relative to masonry. Slotted/slip connections are a must at one or both ends.
 Note: one method for accommodating thermal movement is to use CJs at one or both ends, which reduces wall and lintel effectiveness, but is necessary for differentially moving material.
Vertical wall jamb reinforcement location- generally needs to be one or more cells away from opening
Torsional effects, especially with wide flanges with virtually no torsional capacity
For bearing plates, compatibility with block shapes used
Thermal bridging- architectural challenge with building insulation envelope
Thermal bridging- structural challenge with differential thermal movement between steel and masonry
Consider the thin masonry shells used to cover the steel
\Box Are the masonry shells able to be cut to fit the steel section?
□ No connections are allowed on masonry soaps
Review bearing plate details for steel joist and steel beams perpendicular to masonry walls
Detail must accommodate steel beam movement due to thermal loads during construction
Bearing plates should not bear upon upon masonry face shell in most cases
Consider confined bearing capacity
Masonry bearing plates should not be exposed (never extend to face of masonry)
Conflicts between steel columns and masonry
Does steel column fit into masonry?
Consider a CJ at these locations of steel columns used within masonry
Consider the masonry remaining to cover the steel. Is it required to be cut to fit the steel section? Is remaining masonry durable to building use conditions?
Steel sections should not be encased in grout within masonry, gap should be provided
 Consider: Using masonry piers instead of steel columns to avoid conflicts and support point loads. Often masonry piers have more potential capacity than the steel columns.

REFERENCES

- ¹ Current masonry code is TMS 402/602-16
- ² Based on NCMA TEK 10-2D (2019) or TEK 10-3
- ³ Based on NCMA TEK 10-2D (2019), Figure 2c or Figure 2d (page 3)
- 4 Based on NCMA TEK 10-2D (2019), Figure 2a or Figure 2b (page 3)
- 5 Based on NCMA TEK 10-3
- ⁶ SE INSIGHT: How to Specify Concrete Masonry
- 7 SE INSIGHT: Masonry Partition walls

8 - SE INSIGHT: Efficient Masonry Lintels

MASONRY RESOURCES

- IMI International Masonry Institute imiweb.org
- NCMA National Concrete Masonry Association ncma.org
 - NCMA TEK 10-2D (2019) Control Joints for Masonry Structures Empirical Method
 - NCMA TEK 10-3 (2003) Control Joints for Masonry Structures Alternative Engineered Method
- **BIA** Brick Industry Association <u>gobrick.com</u>
- TMS The Masonry Society masonrysociety.org
 - TMS 402-16 Building Code Requirements for Masonry Structures
 - TMS 602-16 Specification for Masonry Structures

FORSE - FOR Structural Engineering Masonry Resources - <u>http://masonry.forsei.com/technical/se-insights/</u>

- Visit the site to view the current, up to date Masonry Checklist
- SE INSIGHT: How to Specify Concrete Masonry
- SE INSIGHT: Masonry Partition walls
- SE INSIGHT: Efficient Masonry Lintels
- SE INSIGHT: Block Strength Map: <u>masonry.forsei.com/masonry/cmudata/</u>